

PennDOT e-Notification

Bureau of Solutions Management
Highway Applications Division



PSLRFD

No. 023
August 3, 2026

Release of Version 2.17.0.0

The Department's LRFD Prestressed Concrete Girder Design and Rating (PSLRFD) program has been revised as described in the attached "Summary of February 2026 Revisions – Version 2.17.0.0".

The new version has been placed on PennDOT servers for use by the Districts. Consultants and others, who have a current license agreement for **PSLRFD Version 2.16.0.0**, can obtain the updated version by submitting an [Update Request Form](#) along with the **update fee of \$500 for private organizations and \$50 for governmental agencies**. Updates for **PSLRFD Version 2.15.0.0 or earlier** will require an **additional fee**. For update fee details, refer to the [PSLRFD Fee Schedule](#). The update fee is waived for federal and state transportation agencies.

Once payment is received, an e-mail will be sent with download instructions. A valid e-mail address must be provided on the Update Form to receive the download instructions.

Please direct any questions concerning the above to:

Nicole Avalon | AMS Highway Permit Admin (HPA) Engineer Business Analyst
PA Office of Administration | Infrastructure and Economic Development
Bureau of Solutions Management | CAI AMS Highway Admin Team
400 North Street, 5th Floor | Harrisburg, PA 17120
Email: c-navalon@pa.gov
www.oa.pa.gov

Attachment

Archived copies of all previously distributed e-Notifications can be obtained from the PennDOT LRFD and Engineering Programs website at <http://penndot.engrprograms.com/home> and clicking on "e-Notification" and then "Mailing List Archives."

SUMMARY OF FEBRUARY 2026 REVISIONS - VERSION 2.17.0.0

Since the release of PSLRFD Version 2.16.0.0 several revision requests and user requested enhancements have been received. This release of PSLRFD Version 2.17.0.0 contains the following revisions and enhancements.

New Feature

1. A non-composite dead load has been added to the Pedestrian command to allow consideration of changes to non-composite dead load when the bridge has sidewalks. (Request 430)

User Manual Revisions

2. A brief description of rotation distribution factors has been included in User Manual section 3.5. (Request 539)
3. The basic development length modification factor for epoxy-coated bars detailed in User Manual section 6.18.4 has been revised to include clear spacing limits for the 1.5 factor value and the 1.2 factor value for other epoxy-coated bars. Note that this change reflects the criteria that was being used to compute the coating factor. (Request 579)
4. Note 2 of the BDM command in the User Manual has been updated for clarity. (Request 732)
5. The User Manual and Engineering Assistant have been updated to include information about load factor applications and instructions on inputting custom load factors. (Request 753)
6. References to Windows 8.1 as a supported operating system have been removed from Chapter 4 of the User Manual. (Request 770)
7. The stability checks on the First Girder Seated, described in Section 3.25.2 of the user manual, have been updated to reference Section 2.4.4 of AASHTO Guide Design Specification for Bridge Temporary Works (2017) with a safety factor of 1.5. (Request 788)

Input Revisions

8. Additional input checks have been added for the TND command for the tendon longitudinal locations. (Request 755)
9. Input file names starting with either "APRAS" or "apras" are no longer considered to be APRAS runs. (Request 762)

Output Revisions

10. The Detailed Rating Factor report for Strength Moment Ratings now reports the computed rating factors at simple bearings near interior supports. Previously, ratings at these locations were reported as 999.99. Also, ratings factors are now computed for negative live load moment at locations where the total minimum factored moment is positive, and the minimum live load moment is negative. Previously, these ratings were reported as 999.99. (Request 729)
11. Design runs requesting f_c optimization now have new top-level bookmarks that provide greater navigation in the PDF output. (Request 740).
12. The output report for the Service IIIA check of 90% Yield Stress of P/S Strands has been revised to ignore analysis points located between simple bearings at interior supports. Before this change analysis points located between simple bearings would report a Code Failure for analysis points that were not at the interior support. (Request 748)
13. The Overhang Width output report only defaults to print when a valid OVH command has been given by the user. (Request 757).
14. For the debonding strand analysis of I-beam cross sections (PennDOT I-Beams, AASHTO I-Beams, and PA Bulb Tee Beams), a strand pattern visualization will be generated and printed in the output report. (Request 761)
15. For the debonding strand analysis of I-beam cross sections (PennDOT I-Beams, AASHTO I-Beams, and PA Bulb Tee Beams), warnings have been added to the report when the specified debonding pattern requires debonding of the outer-most strands in the full width portion of the bottom flange or strands within the horizontal limits of the web. (Request 771)

Design Revisions

16. Debonded strand design runs now include an explicit minimum reinforcement check. Previously, on rare occasions a debonded strand design run would report a failure for the Minimum Prestressing Steel Check. (Request 064).
17. Multiple shear areas (horizontal and vertical) can be specified for shear design runs within a span. Previously, multiple areas could be entered but only the first area was being used. (Request 704).
18. The design spacing for horizontal stirrups in box beams has been limited to a minimum of 6 inches to agree with Standard BD-661M Box Beam Reinforcement Details. Box beam designs requiring less than 6 inch spacing of horizontal stirrups will fail for the current beam size. A horizontal stirrup area of 0.62 in^2 (2 - #5 bars) does not result in a warning message in the Shear Reinforcement for Drawings output. (Request 746)

19. The minimum number of strands in the bottom row for adjacent boxes with debonded strands has been changed to 8 to allow 2 strands to be debonded when only a total of 8 strands are required for the design. Previously, designs requiring only 6 strands in the bottom row could fail the debonding design. (Request 760)

Engineering Assistant Revisions

20. The figure names used in Engineering Assistant's tabs and descriptions have been updated to be consistent with the figure names used in the User Manual. (Request 763)

Programmer Revisions

21. The tendon location from the right simple bearing no longer considers zeroes input by the user in tendon location calculations. Previously, the program would determine tendon locations using any value entered, including zero. This would set the tendon location to a distance outside of the span length. The program now disregards tendon locations of zero for end block calculations. (Request 774)
22. The tolerance used to determine if a debonding location corresponds to crack control debonding has been changed to +/- 0.5 inches. Previously, the tolerance was 0.0001 feet or 0.0012 inches. (Request 775).
23. The program has been revised to use Visual Studio 2022 and Intel Fortran Compiler Classic 2021.12.0. (Request 777)
24. The program has been made compatible with the forthcoming release of Engineering Assistant v3.0.0.0. Changes include:
- 1) switching image formats from Enhanced Metafile (.emf file) to Portable Network Graphic (.png file) required by the new Engineer Assistant,
 - 2) associating a Tab Name with the current 3 letter tab abbreviation,
 - 3) resolving issues when setting floating-point precision on 64-bit systems. (Request 784)